

PHYSICAL MODEL APPROACH OF CRIB WALLS TO IMPROVE THE RIVER SLOPE STABILITY

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Abstract

This study aims to determine the pattern of crib walls collapse at river bends and the effect of the slope angle of the crib walls not escaping the water as a crib walls collapse control as well as sediment control in channels with various bend angles. The model was made in a channel 25×20 cm, a river length of 600 cm. Sediment distribution from fine sand that is not homogeneous and the flow is clear (clear water). The angle and distance of the crib walls installation are varied. The crib walls used in this experiment were 5 cribs with a tilt angle of 30° and 60° . Each treatment was observed with parameters related to erosion and sedimentation in the river bank bends, including velocity (v), time (t), depth of erosion (de), sedimentation (ds). The dimensional analysis method is used to see the relationship between dimensionless parameters with the Langhaar method. The results showed that the maximum relative sedimentation (ds_5/t) max for the crib walls angle of 30° occurred in the fifth crib walls of 0.025 at a relative speed (v/t) of 0.06. While the maximum relative erosion depth (ds_3/t) max for the tilt angle of 30° crib walls occurs in the first grout, which is 0.012 at a relative speed (v/t) of 0.0042. At the angle of 60° crib walls, there is a maximum relative erosion depth (de_3/t) of 0.082 at a relative speed (v/t) of 0.006 on the third crib walls. The increasing of the relative velocity (v/t) the greater the value of the relative erosion depth (ds/t).

Keywords: Crib walls; scale models; erosion and sedimentation; slope stability

1. INTRODUCTION

In the era of globalization, the development of urban areas is increasingly developing, the more problems that not only bring good effects but also have bad effects on natural and environmental conditions. The river is one of the aquatic ecosystems that is influenced by many factors, both in natural activities and human activities in the watershed. In the management of a watershed, it is necessary to pay attention to the water body of the watershed. Incorrect watershed management will have an impact on the sustainability of river water bodies, namely very highwater discharge fluctuations and reduced river capacity (Paimin et al, 2012).

For physical modelling, it is usually done by reducing the various variables, namely by giving a scale (n) on each of these variables. Meanwhile, the scale of the various variables or parameters can be determined based on the relationship between the parameters expressed in dimensionless numbers, such as Reynold numbers, Froude numbers and so on. In addition to determining the relationship between scales, this dimensionless number can also be used to describe research results, thus the results of these studies can be generalized (Holdani Kurdi, 2019).

To determine the dimensionless number can be done by dimensional analysis (Holdani Kurdi, 2019). Dimensional analysis to determine the dimensionless number there are several ways, including by:

- Basic echelon matrix
- Buckingham (phi.theorem)
- Rayleigh
- Stepwise, dan
- Langhaar

If the hydraulic phenomenon/event can be explained by n parameters P_i with $i = 1, 2, 3, \dots, n$ and if the parameter is composed of m principal elements, then the product of dimensionless numbers that can be derived number $(n-m)$. For hydraulic engineering purposes, there are usually 3 main elements, namely: mass (M), length (L), and time (T).

$j = P_1^{k_1} \cdot P_2^{k_2} \cdot P_3^{k_3} \cdot \dots \cdot P_n^{k_n}$, where

j = product of dimensionless numbers with $j = 1, 2, 3$

If P_i has dimension M, then the dimensions can be written as follows:

$= (M^{\alpha_1} L^{\beta_1} T^{\tau_1})^{k_1} * (M^{\alpha_2} L^{\beta_2} T^{\tau_2})^{k_2} * \dots * (M^{\alpha_n} L^{\beta_n} T^{\tau_n})^{k_n}$
 $= [M^{(\alpha_1 k_1 + \alpha_2 k_2 + \dots + \alpha_n k_n)}] [L^{\beta_1 k_1 + \beta_2 k_2 + \dots + \beta_n k_n}] [T^{\tau_1 k_1 + \tau_2 k_2 + \dots + \tau_n k_n}]$ is a dimensionless number if:

$$\alpha_1 k_1 + \alpha_2 k_2 + \dots + \alpha_n k_n = 0$$

$$\beta_1 k_1 + \beta_2 k_2 + \dots + \beta_n k_n = 0$$

$$\tau_1 k_1 + \tau_2 k_2 + \dots + \tau_n k_n = 0$$

the coefficients α_i , β_i and τ_i can be known from the related P_i parameters.

In connection with this problem, in this study a laboratory experiment was carried out to determine the pattern of cliff collapse at river bends and the effect of laying non-permeable cribs as a control for cliff collapse as well as controlling sediment in a channel with a bend angle of 120° . Simulation of the installation of cribs was also carried out on variations in flow discharge at river bends. The model is made in a channel of 40×40 cm and the length of the river is 1,280 cm and the length of the bend is 150 cm. Sediment of fine sand that is not homogeneous and flows in clear conditions (clear water). The angle and distance of the crib installation varies (Syarifudin, A et al, 2018).

2. RESEARCH METHODS

2.1. Research sites

This research was conducted in the Hydrology and Hydraulics laboratory of Bina Darma University Palembang as shown in Figure 1 (Syarifudin, A et al, 2019).

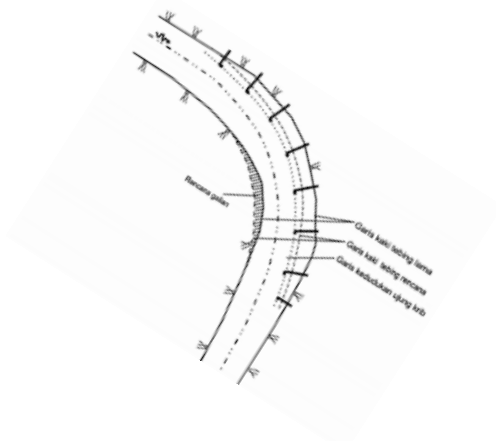


Figure 1: Crib walls design model (Syarifudin, A, 2020)

2.2. Research Stages

The research stages are divided into two, namely:

- Physical research, which is carried out in laboratory experiments to observe and record existing phenomena.
- Hypothetical and analytical research, which was conducted to find the relationship and the variables that influence it.

2.3. Research Materials and Tools

The materials used in this study include:

- Sand with a diameter of 0.075 mm to 2.36 mm, is considered a sedimentary material which was previously analyzed by sieve to obtain a uniform grain diameter (d_s) from river material.
- Water, as a medium for moving sedimentary material flows in the channel,

The tool used in this research is a hydraulics laboratory facility at Bina Darma University (Syarifudin, A, 2019).

The specifications of the tool are as follows:

River model with its turns:

- Wall material: made of ordinary mixed cement
- Effective length: 600 cm
- Width: 25 cm
- Depth: 20 cm

Measuring scour depth

- Meter, to measure the location of scour.
- Photo camera to take pictures during experiments
- Video recorder to record the execution of the experiment

2.4. Materials and Tools

The materials and tools used in this study are as shown in table 1 below.

Table 1 List of the Research Tools and Materials

No.	Tools name	Amount	Uses
1.	River scale model	1 set	A tool for writing data recording results
2.	Pump	1 unit	Assists the movement of flow in the model
3.	Crib walls model	3, 5 and 7 unit	Simulation tool
4.	River bed materials	Sieve analysis result	Simulation material
5.	Water	Suitable for storage	Flow simulation

2.5. Research Stages

In accordance with the research objectives, the following stages are required:

- The first stage is to collect references from journals, books, and other secondary data sources.
- The second stage, conducting a field orientation survey to obtain the current (existing) field conditions, taking photos of the field (site) so that it can be used as initial research data.
- The third stage is to design a river with a model scale from prototype to model with a maximum storage capacity of 1000 liters, consisting of 2 circulation tanks located upstream and downstream of the river model with dimensions of 500 cm long, 20 cm wide with a wall slope ratio of 1: 0.005.
- The fourth stage, conducting initial simulation trials to see the readiness of the river model and calibrating so that the model is in accordance with the conditions from prototype to model.
- The fifth stage is to test the model by placing the sediment base material from the sieve analysis by taking the average diameter (d_{50}) with the assumption that the base material corresponds to that in the river prototype. Followed by the installation of the position of the crib model at the bend of the river with a certain distance of 5 crib walls models
- The sixth stage, conducted a trial with a running time of 60 minutes, with every 15 minutes good observations were made with 3 groin models, then 5 crib walls models and 7 groin models. Observations and recordings of erosion and sedimentation patterns were carried out in each scenario of the installation of the crib walls model.
- The seventh stage, discusses the results of observations that occur in the crib walls model and makes research conclusions and provides suggestions for further research by other studies.

3. RESULTS AND DISCUSSION

3.1. Dimensional Analysis

Dimensional analysis in this study uses Langhaar's theorem, this theorem is considered more in line with current conditions and in accordance with research because the parameters are relatively few. The steps of dimensional analysis are as follows:

1. In the problem formulation it is stated that the parameters that affect the erosion around the groin include the angle of the crib walls slope (α), water depth (h), flow velocity (v), erosion depth (de), time (t) and acceleration gravity (g), and the density of water (ρ_w).
2. The parameters are grouped into:
 - a. Dependent parameter: v
 - b. Parameters changed during the experiment: ds , h , and t
 - c. Other parameters: α , g , ρ_w
3. The prices of α_1 , β_1 and γ_1 are determined as shown in table 2 below:

Table 2 Determination of Dimensional Analysis

Group	1	2			3			Note
Parameter	v	ds	h	t	α	ρ	g	
M	0	0	0	0	0	1	0	α_1
L	1	1	1	0	0	-3	1	β_1
T	-1	0	0	1	0	0	-2	γ_1
	k_1	k_2	k_3	k_4	k_5	k_6	k_7	k_i

Determination of dimensionless numbers as in table 3.

Table 3 Dimensionless Numbers

k_i	k_1	k_2	k_3	k_4	k_5	k_6	k_7
Parameter	v	d_s	h	t	α	ρ	g
π_1	1	0	0	-1	0	0	0
π_2	0	1	0	-1	0	0	0
π_3	0	0	1	-1	0	0	0
π_4	0	0	0	1	0	0	-2

This results in the following dimensionless equation:

$$\pi_1 = v/t \quad (1)$$

$$\pi_2 = de/t \quad (2)$$

$$\pi_3 = h/t \quad (3)$$

$$\pi_4 = t/2g \quad (4)$$

$$(h/t) \times (t/2g) = 2gh = v$$

$$f(v/t; de/t; v) = 0 \quad (v \approx 0)$$

$$(v/t) = f(de/t) \text{ focus on erosion around the cribs}$$

3.2. 5 (five) crib walls models with an slope angle of 60°

In conditions where there are 5 (five) crib walls at the bend of the river with a slope angle of 60° as shown in Figure 1.



Figure 2: The Placement of the crib walls with slope angle of 60°

Table 4 The Results of Analysis of the Scouring Depth at the Crib walls Slope Angle of 60°

V	(t)	Non dimensional parameter					
		V/t	de ₁ /t	de ₂ /t	de ₃ /t	de ₄ /t	
0.03	5	0.006	0.020	0.060	0.080	0.018	0.018
0.03	5	0.006	0.020	0.060	0.080	0.018	0.018
0.03	5	0.006	0.020	0.060	0.080	0.018	0.040
0.03	5	0.006	0.020	0.060	0.080	0.018	0.040
0.03	5	0.006	0.020	0.060	0.100	0.018	0.060
0.03	10	0.003	0.001	0.001	0.009	0.010	0.040
0.03	10	0.003	0.001	0.001	0.009	0.010	0.050
0.03	10	0.003	0.001	0.003	0.030	0.009	0.050
0.03	10	0.003	0.002	0.003	0.030	0	0.050
0.03	10	0.003	0.003	0.003	0.030	0.020	0.050
0.03	15	0.002	0.001	0	0	0.000	0.007
0.03	15	0.002	0.001	0	0.006	0.005	0.013
0.03	15	0.002	0.001	0.006	0.006	0.005	0.013
0.03	15	0.002	0.001	0	0.006	0.005	0.013
0.03	15	0.002	0.001	0	0.006	0.007	0.005

3.3. Simulation Results

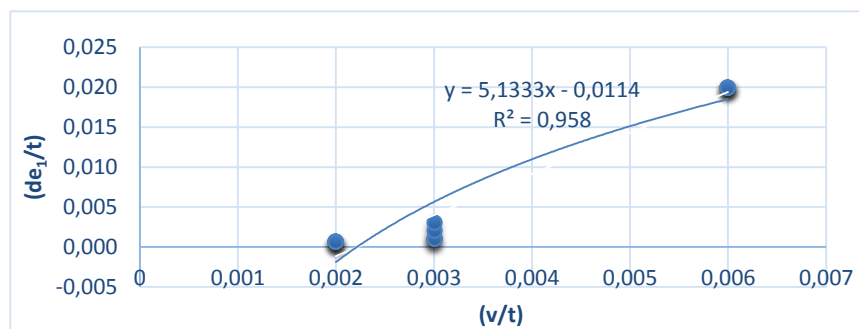


Figure 2: The relationship between (v/t) and (de₁/t) of the crib walls with slope angle of 60°

In Figure 2, If the time taken is 60 minutes, then the velocity v that occurs is 0.12 cm/sec, then with a velocity v of 0.12 cm/sec taken as a benchmark calculation, the amount of sedimentation that occurs is 0.648 cm or with a scale of 1:100, then the amount of sedimentation is 64.8 cm in the model or 0.648 m in the prototype (site).

Then erosion begins to occur at a certain distance and increases to the maximum erosion depth at a relative velocity (de_1/t) of 0.006, which is 0.02. Likewise, the erosion depth is 72 cm in the model or 0.72 m in the prototype (site).

The value of $R^2 = 0.958$ or $R = 0.978$ and this means that 97.8 percents there is a suitable value between the depth of erosion with the velocity parameter v and time t on the cribs placement with an angle of 60° .

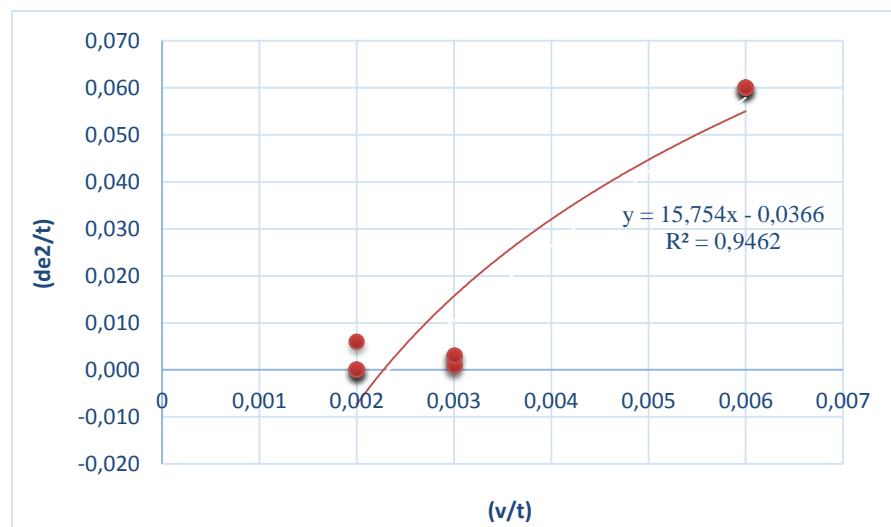


Figure 3: The relationship between (v/t) and (de_2/t) of the crib walls with slope angle of 60°

Almost the same as what happened in the second crib walls, as seen in Figure 3. It can be seen that the maximum relative erosion depth (de_2/t) is 0.06 at a relative velocity (v/t) of 0.006. and then the value of $R^2 = 0.946$ or $R = 0.972$, this means that 98.59 percents there is a suitable value between the depth of erosion with the velocity parameter v and time t on the cribs placement with an angle of 60° .

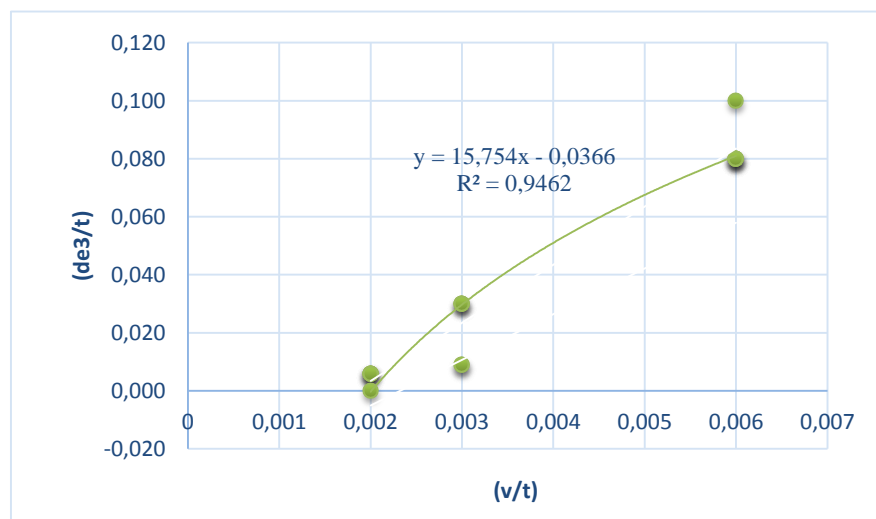


Figure 4: The relationship between (v/t) and (de_3/t) of the crib walls with slope angle of 60°

Figure 4. The third crib walls shows that the relative velocity (v/t) increases significantly in a straight line or linear trend, namely the greater the maximum relative depth value (de_3/t)max of 0.08.

The value of $R^2 = 0.946$ or $R = 0.972$, this means that 98.59 percents there is a suitable value between the depth of erosion with the velocity parameter v and time t on the groynes placement with an angle of 60° . The same like The relationship between (v/t) and (de_2/t) of the 60° cribs slope angle. (figure 5).

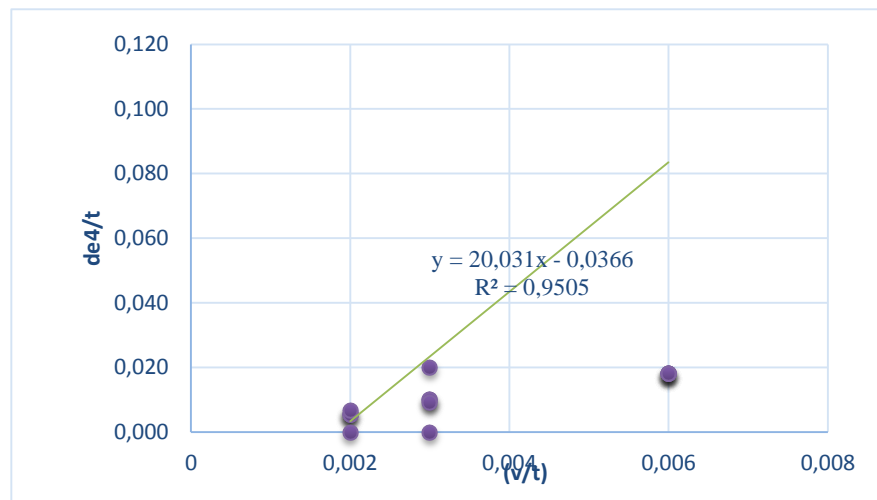


Figure 5: The relationship between (v/t) and (de_4/t) of the crib walls with slope angle of 60°

In contrast to the graph in figure 5. where the greater the relative velocity (v/t) , the more it shows a decreasing trend in the relative depth of erosion (de_4/t) .

This is because the longer the flow time (t) , the velocity v will decrease and be stable and there is no increase in erosion at river bends.

The value of $R^2 = 0.950$ or $R = 0.974$, this means that 97.4 percents there is a suitable value between the depth of erosion with the velocity parameter v and time t on the cribs placement with an angle of 60° .

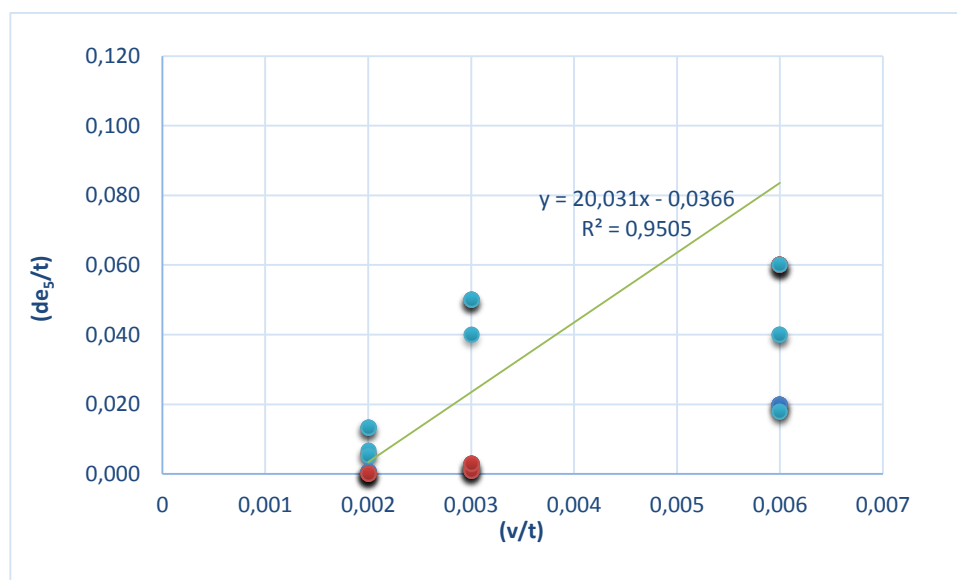


Figure 6: The relationship between (v/t) and (de_5/t) of the crib walls with slope angle of 60°

Figure 6 shows that there has been a maximum relative depth at the fourth crib walls (ds_4/t) , namely at a relative velocity (v/t) of 0.004 with a maximum relative depth of erosion (de_5/t) of 0.06. After that, there was a decrease in the relative depth of erosion (de/t) proportional to the relative velocity (v/t) that occurs.

It is the same with the value of $R^2 = 0.950$ or $R = 0.974$, this means that 97.4 percents there is a suitable value between the depth of erosion with the velocity parameter v and time t on the crib walls placement with an angle of 60° .

4. CONCLUSION

At the slope angle of 60° , the maximum relative erosion depth (d_e/t) is 0.082 at a relative velocity (v/t) of 0.006 at the third crib walls. So, if the velocity is 1 m/sec with 1 hours, the **erosion** occurs 0,029 cm in the model or **2.90 m** (in site) with scale of 1:100 and with $R = 0,974$ or means that 97.4 percents there is a suitable value between the depth of erosion with the velocity parameter v and time t on the crib walls placement with an angle of 60° .

REFERENCES

- Achmad Syarifudin and Dewi Sartika., (2019). A Scouring Patterns Around Pillars of Sekanak River Bridge, Journal of Physics: IOP Conference Series, volume 1167, 2019, IOP Publishing
- Achmad Syarifudin., (2018). Hidrologi Terapan, Andi Offset, Yogyakarta
- Achmad Syarifudin., (2018). Drainase Perkotaan Berwawasan Lingkungan, Andi Offset, Yogyakarta
- Ahmed SMU, Hogue MM and Hossain S., (1992). Floods in Bangladesh: A Hydrological Analysis, Final Report R01/92, Institute of Flood Control and Drainage Research (IFCDR), Bangladesh University of Engineering and Technology (BUET), Dhaka, pp.1-5
- Achmad Syarifudin., (2017). The influence of Musi River Sedimentation to The Aquatic Environment DOI: 10.1051/mateconf/201710104026, MATEC Web Conf, 101, 04026, [published online 09 March 2017]
- Cahyono Ikhsan., (2017). The effect of variations in flow rate on the bottom of an open channel with uniform flow, Civil Engineering Media.
- Chow V.T., D.R. Maidment and L.W. Mays., (1988), Applied Hydrology. Mc. Graw Hill co. Department of Public Works., Guidance for Landslide Management Planning, SKBI - 2.3.06., 1987, PU Publication Agency Foundation
- Directorate General of Human Settlements, Ministry of Public Works., (2010). Procedures for Making Retention Ponds and Polders With Main Channels. Directorate General of Human Settlements, Ministry of Public Works. Jakarta.
- Department of Public Works., Guidance for Landslide Management Planning, SKBI - 2.3.06., (1987). PU Publication Agency Foundation Islam MZ
- Holdani Kurdi et al., (2019). Model Hidrolika, Lambung Mangkurat University Press
- Istiarto., (2012). Teknik Sungai, Transpor Sedimen, Gadjahmada University, Yogyakarta
- Istiarto., (2012). Teknik Sungai, Gadjahmada University, Yogyakarta
- Mc. Cuen R.H., (1982), A Guide to hydrologic analyses using SCS methods. Prentice Hall Publication
- Madhu Sudan Acharya., (2018). Analytical Approach to Design Vegetative Crib Walls, Geotech Geol Eng (2018) 36:483–496
- Okubo K, Muramoto Y, and Morikawa H., (1994). Experimental Study on Sedimentation over the Floodplain due to River Embankment Failure. Bulletin of the Disaster Prevention Research Institute, Kyoto University, 44 (2), pp. 69-92
- P. Manasa1 et al., (2021). Crib retaining wall, International Journal for Modern Trends in Science and Technology · July 2021
- Paimin et al., (2012). Watershed Management Planning System, Research and Development Center for Conservation and Rehabilitation (P3KR), Bogor, Indonesia
- Robert. J. Kodoatie, Sugiyanto., (2002). Flood causes and methods of control in an environmental perspective, Yogyakarta
- Syarifudin A, HR Destania., (2020). IDF Curve Patterns for Flood Control of Air Lakitan river of Musi Rawas Regency, IOP Conference Series: Earth and Environmental Science Volume 448, 2020, The 1st International Conference on Environment, Sustainability Issues and Community Development 23 - 24 October 2019, Central Java Province, Indonesia

Van Rijn, L.C., (2007). Unified View of Sediment Transport by Currents and Waves II: Suspended Transport. *Journal of Hydraulic Engineering*, Vol. 133, Issue 6, pp. 668-689.

Wan shun Zhang et al., (2014). Modeling Sediment Transport and River Bed Evolution in River System, *Journal of Clean Energy Technologies*, Vol. 2, No. 2, April 2014

Yang, C.T., (1996). *Sediment Transport: Theory and Practice*. McGraw-Hill, Singapore, 1996.

Biodata

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